



Impact of Pore Fluid Compressibility on Soil Cyclic Behavior

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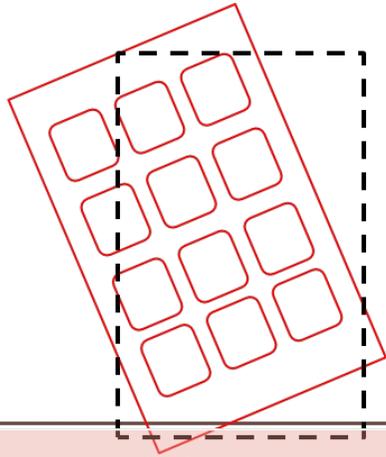
Outline

- Introduction and motivation
 - ✓ Cyclic behavior of loose material under cyclic loading - soil liquefaction
 - ✓ Relationship between pore fluid compressibility and saturation degree
 - ✓ Research motivation
- Material, apparatus, and test procedure
- Results
- Conclusions

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Rupture des fondations Foundation failure



Couche liquéfiée
Liquefied layer



Niigata 1964



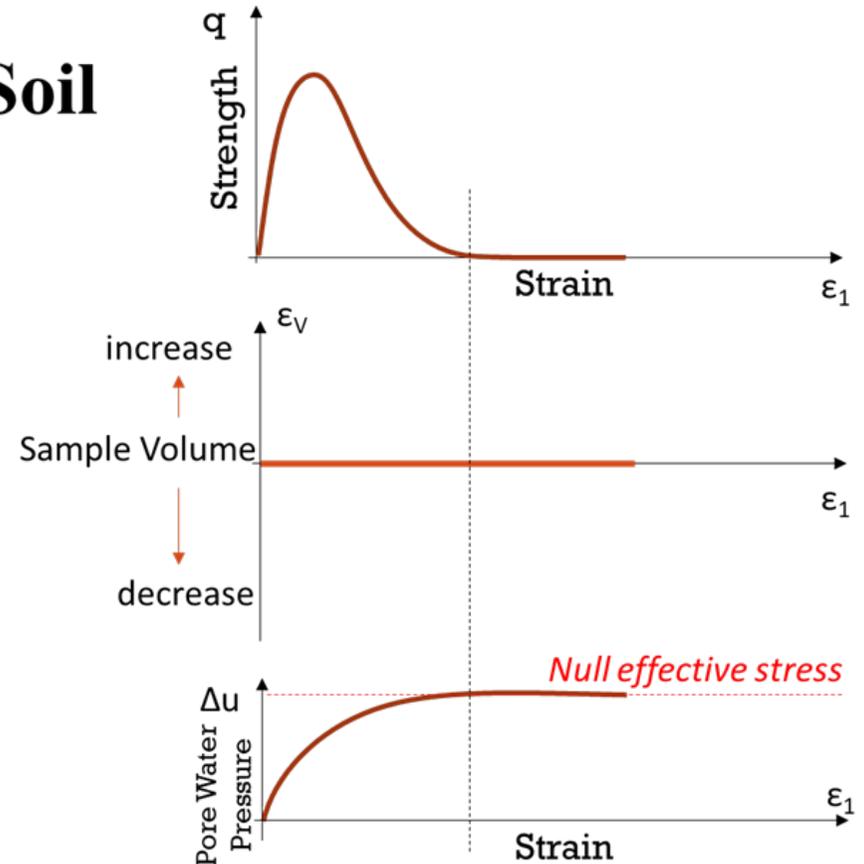
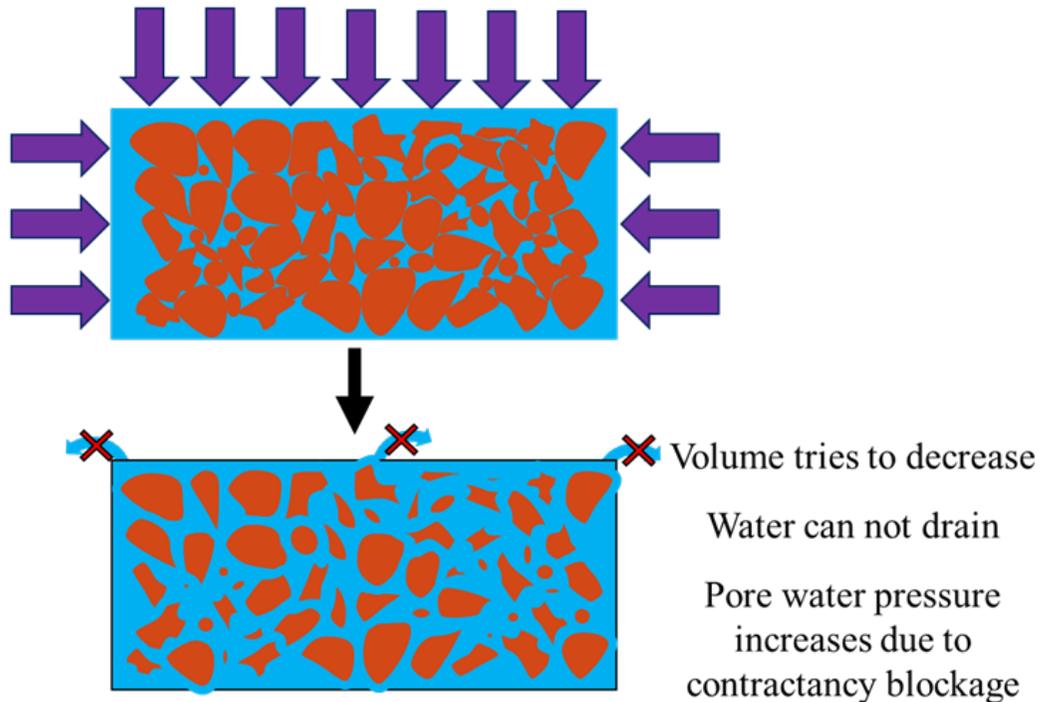
Izmit 1999



Yun Tsui building, Taiwan
2018

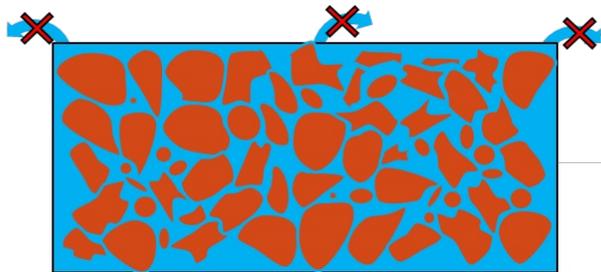
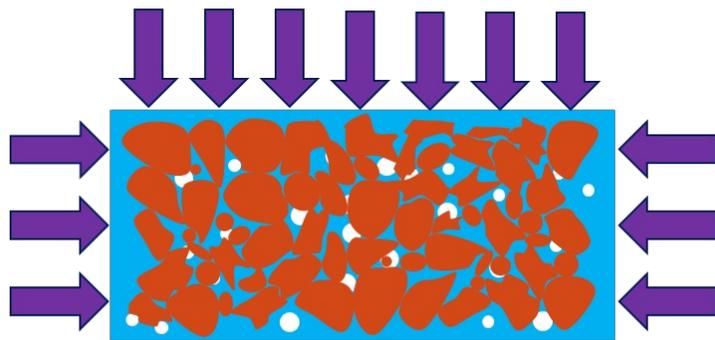
Liquefaction - Fully Saturated

Undrained Loose Soil



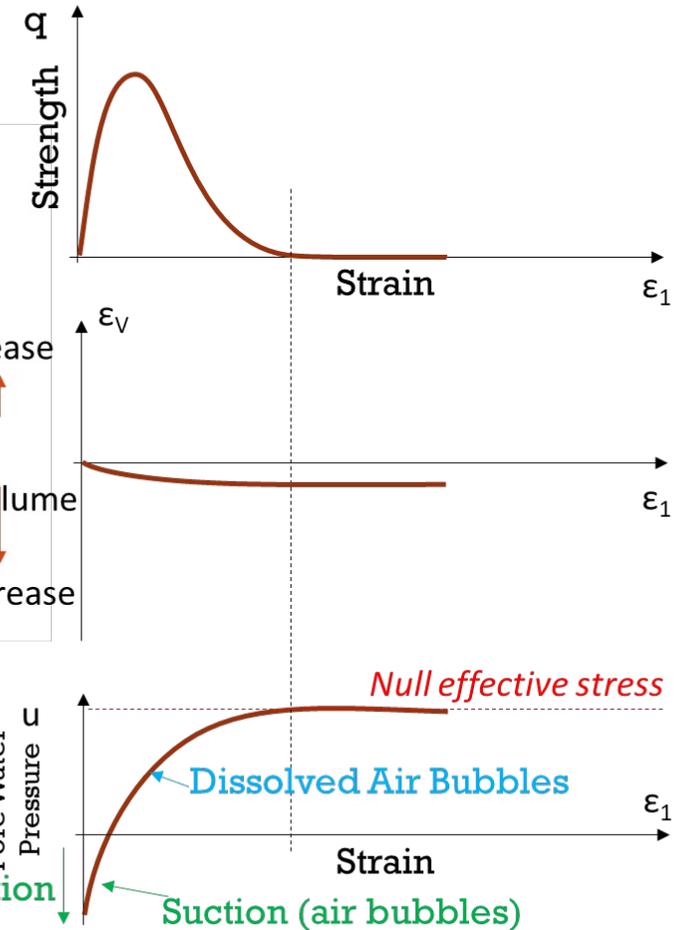
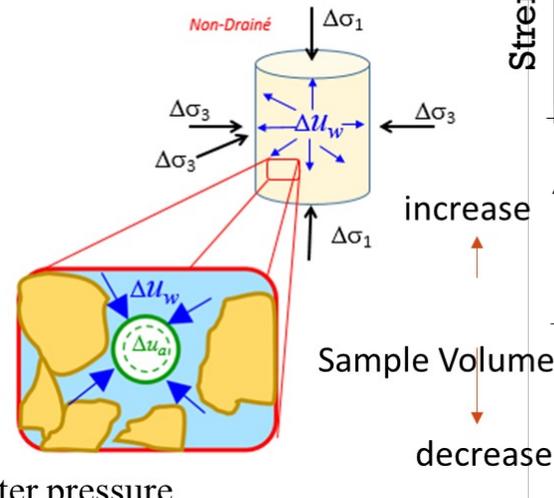
Liquefaction - Unsaturation

Undrained Unsatrated Loose Soil

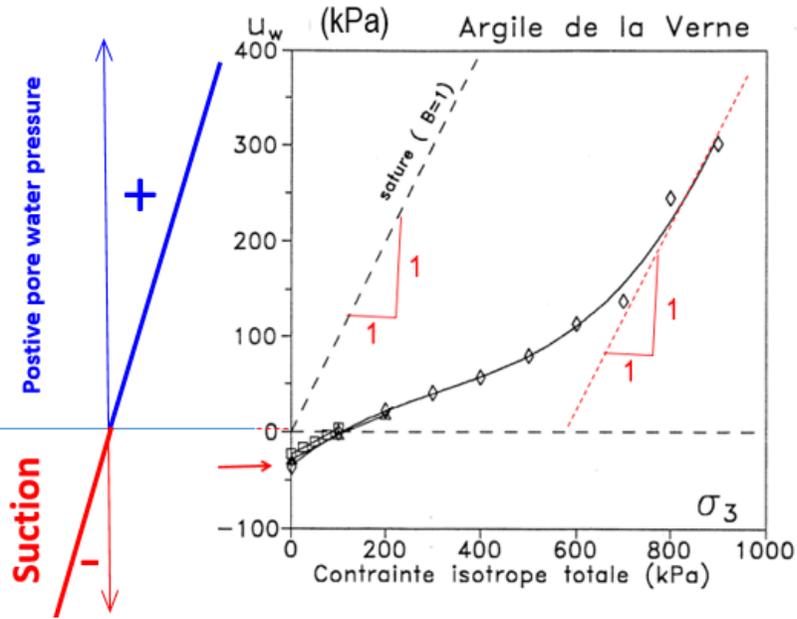


Pore water pressure increases but not significantly

Air takes some of the pressure and dissolve in water

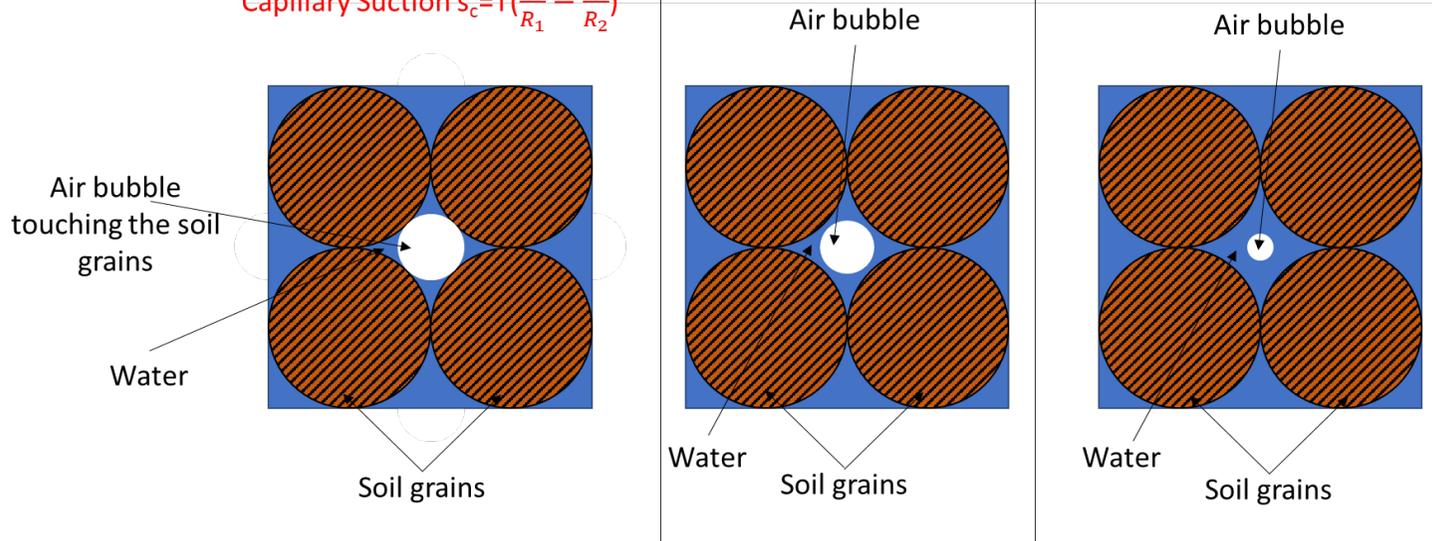


Relationship between saturation degree and suction



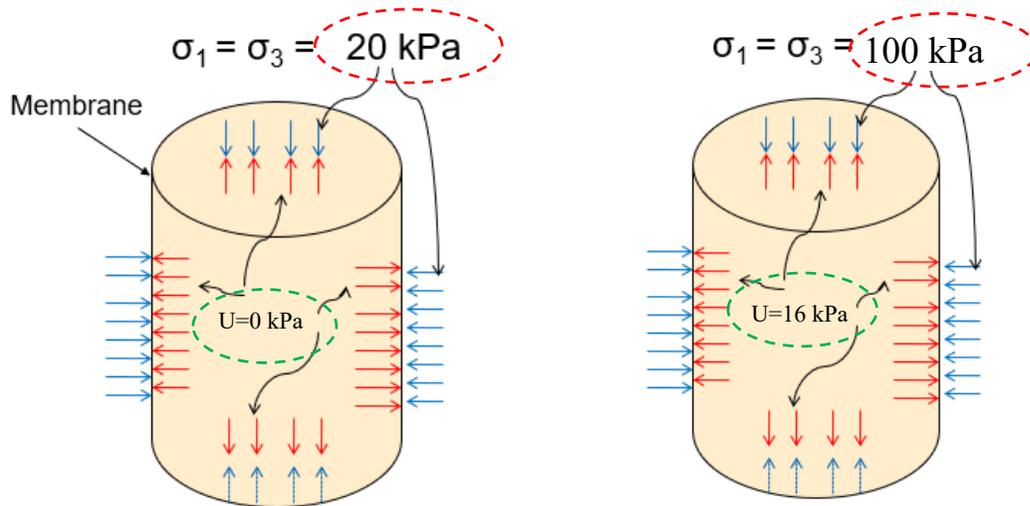
<p>$s_s=0$ $s_c=U_a-U_w \neq 0$ Capillary Suction $s_c \neq 0$</p> <p>Effect of surface suction</p> <p>$S_r \approx 80\%$</p>	<p>$s_c = s_s = 0$ $U_a = U_w$</p> <p>No effect of capillary Suction and spherical suction</p> <p>$S_r \approx 95\%$</p>	<p>$s_s = U_a - U_w \neq 0$ Capillary Suction $s_c = 0$</p> <p>Effect of surface tension</p> <p>100%</p>
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Capillary Suction $s_c = T \left(\frac{1}{R_1} - \frac{1}{R_2} \right)$



Pore fluid compressibility evaluation

B check test



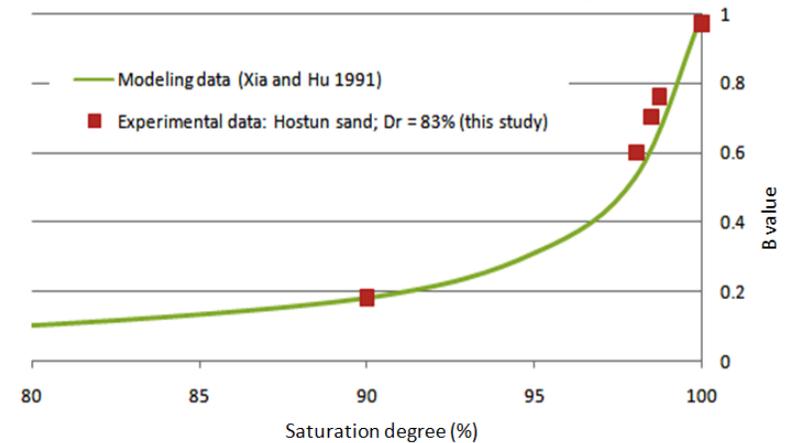
Skempton's parameter

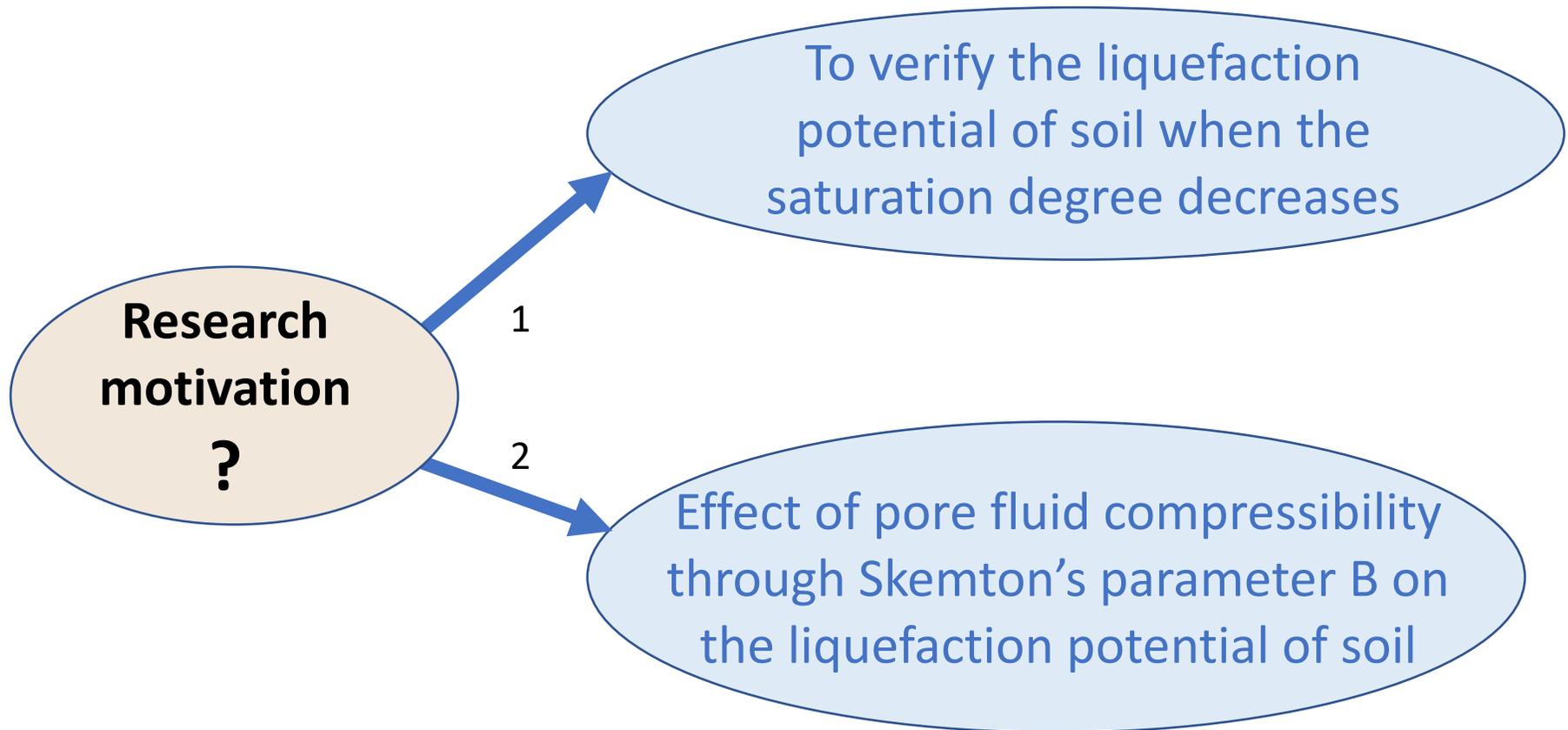
$$B = \frac{\Delta u_w}{\Delta \sigma_3} = \frac{1}{1 + n \frac{K_s}{K_{aw}}}$$

$$B = \frac{16 \text{ kPa}}{80 \text{ kPa}} = 0.2$$

Bulk modulus of Pore fluid

B – Sr relationship



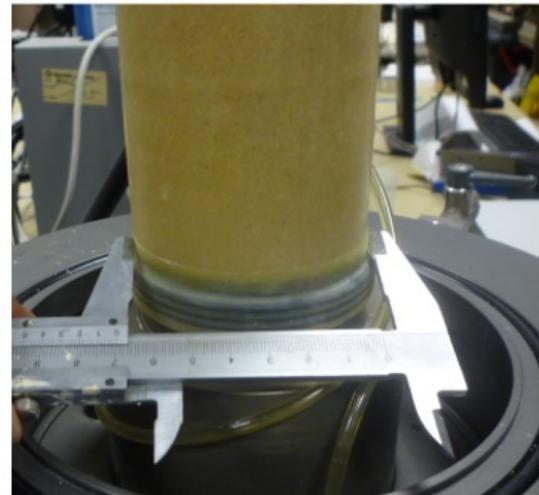
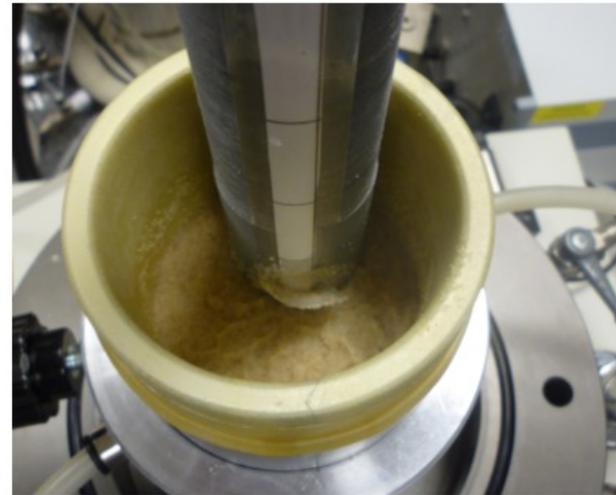


Outline

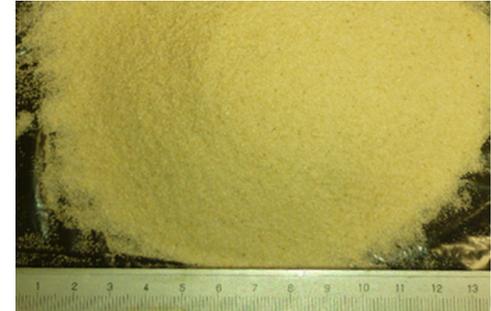
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Material, apparatus, and test procedure.

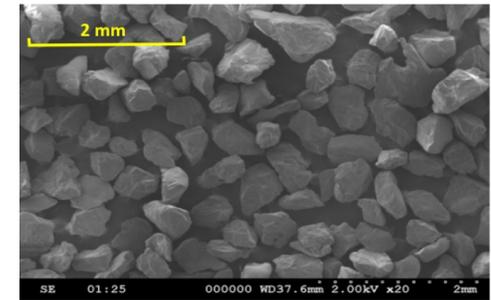
- Material and sample preparation



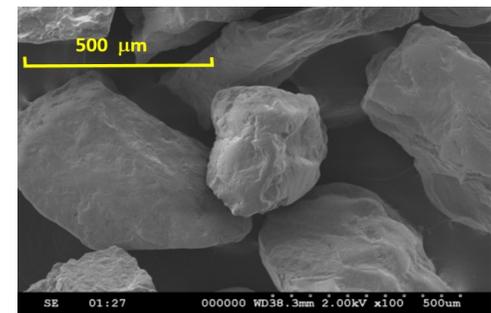
Fine clean sand



uniform grain size



Shape of edges



RF Hostun sand at different scales

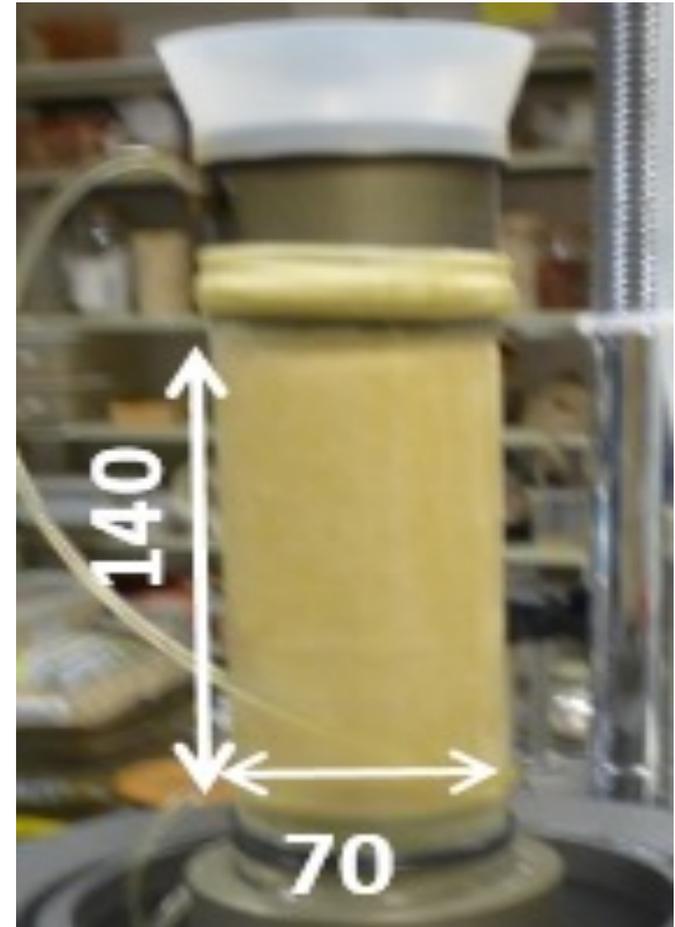
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Material, apparatus, and test procedure.

- Test procedure: sample preparation

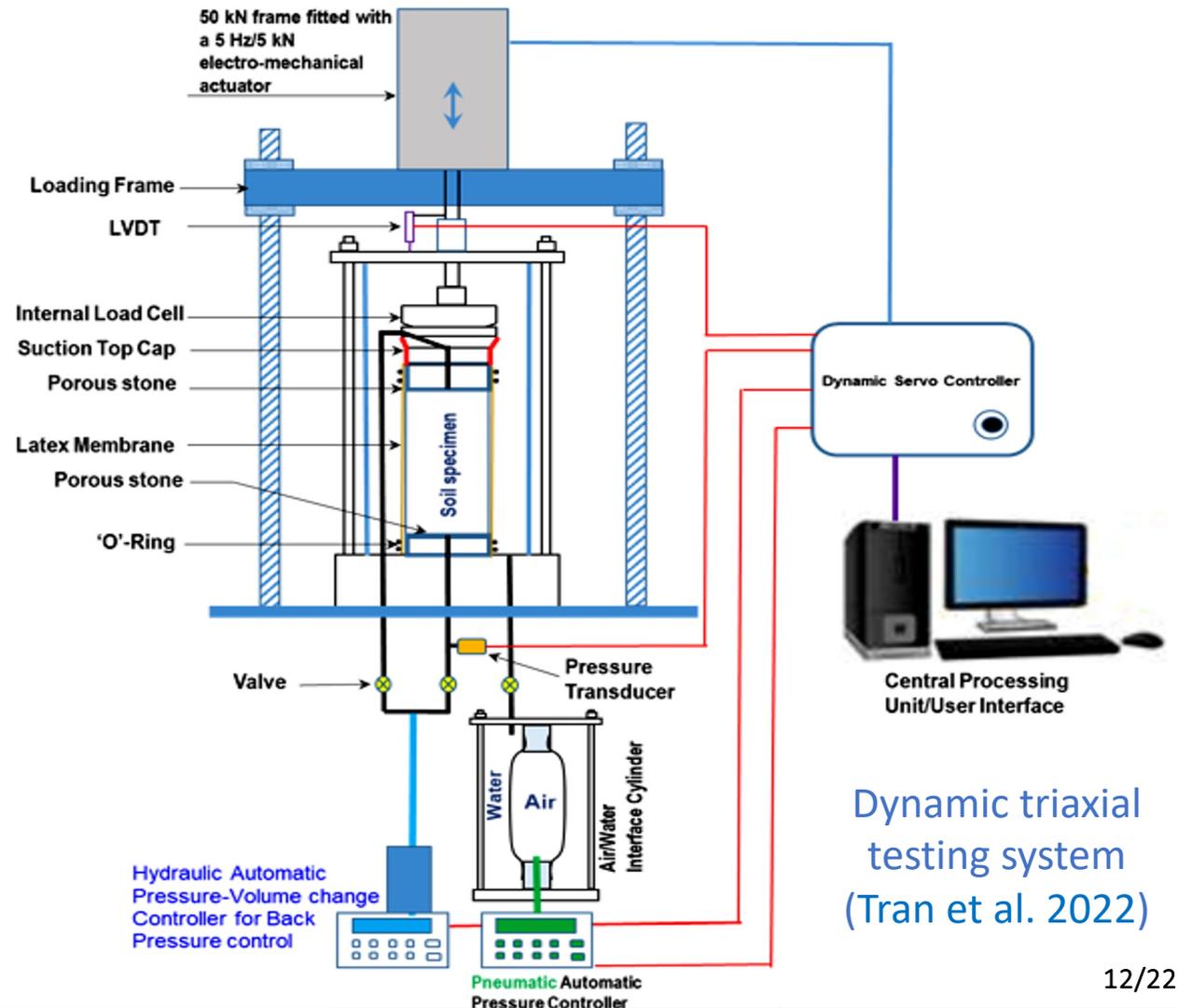
Diameter (mm)	70
Height (mm)	140
Initial Water Content W (%)	8
Initial degree of saturation S_r (%)	30%
Relative density D_r	80%

Test	B	S_{r0} (%)
1	0.97	100
2	0.22	95
3	0.056	86



Material, apparatus, and test procedure.

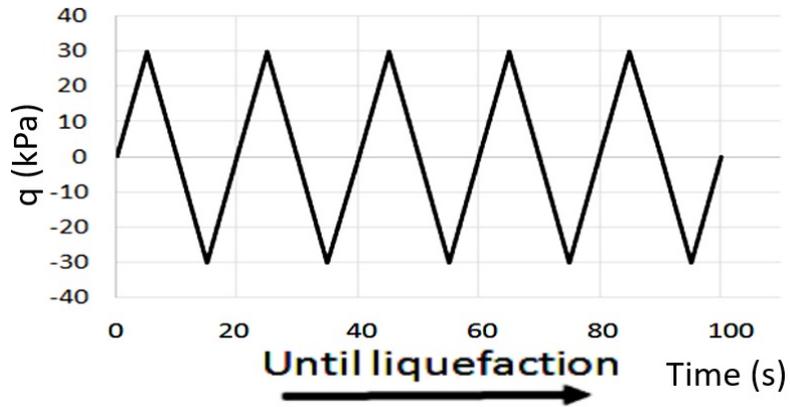
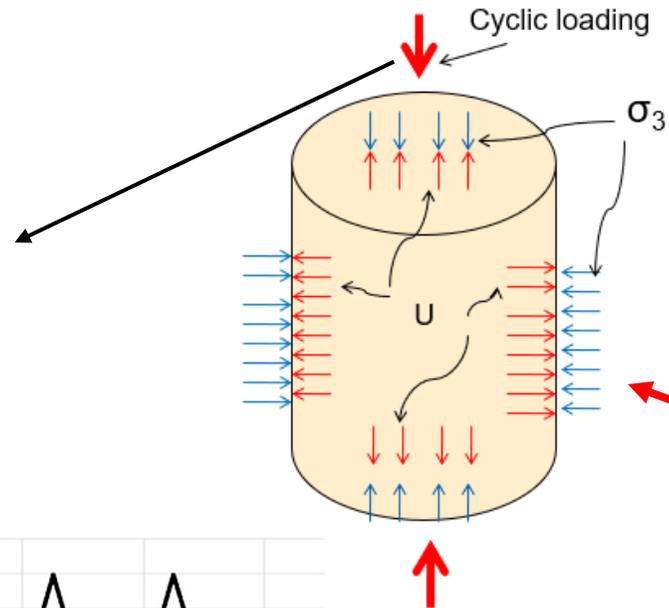
- Apparatus



Material, apparatus, and test procedure.

- Test procedure: cyclic loading

Deviator load



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Unsaturated Sample

Initial state
Skempton's parameter
 $B = 0.2$
-> The sample was
Unsaturated



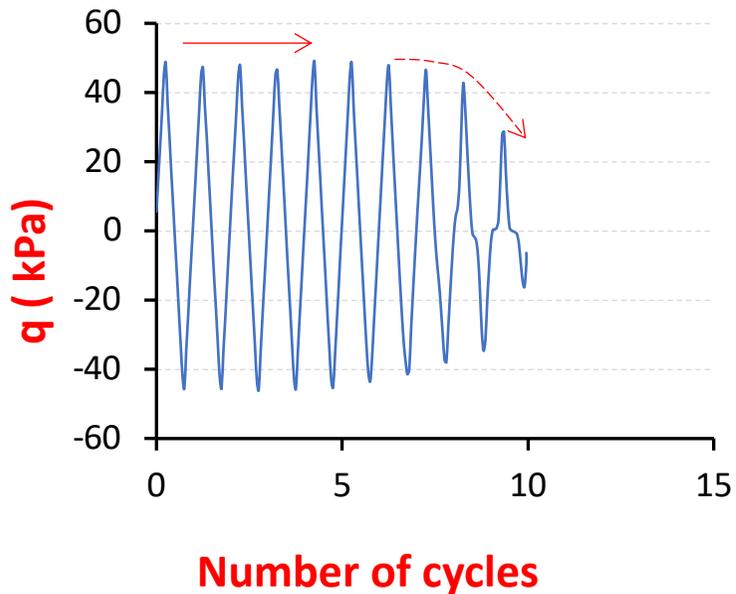
Final state
-> The sample was
Liquefied



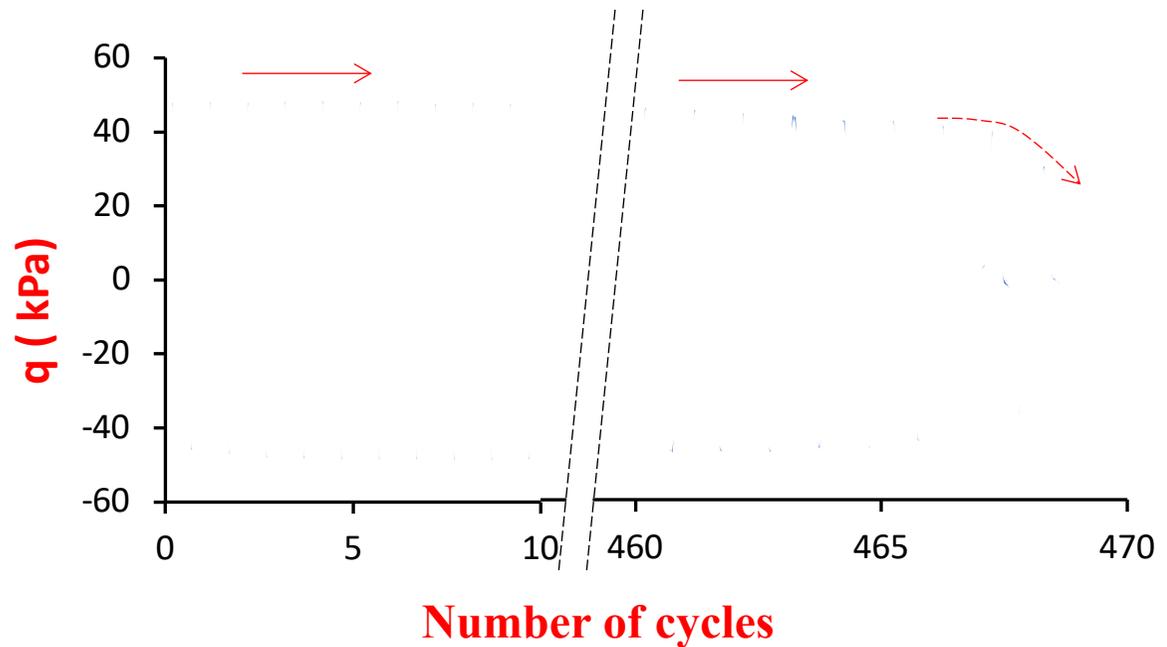
The sample before and after liquefaction

Results

Saturated sample
 $B = 0.97$

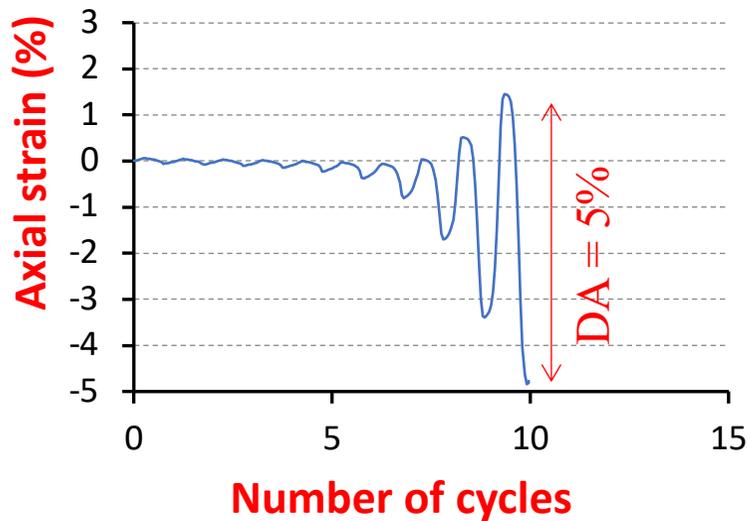


Unsaturated sample
 $B = 0.2$

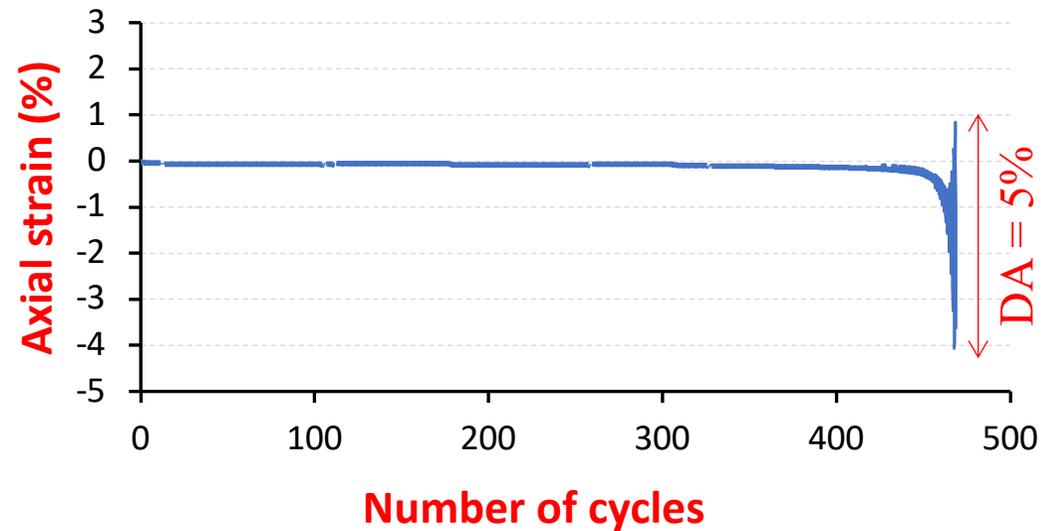


Results

Saturated sample
 $B = 0.97$



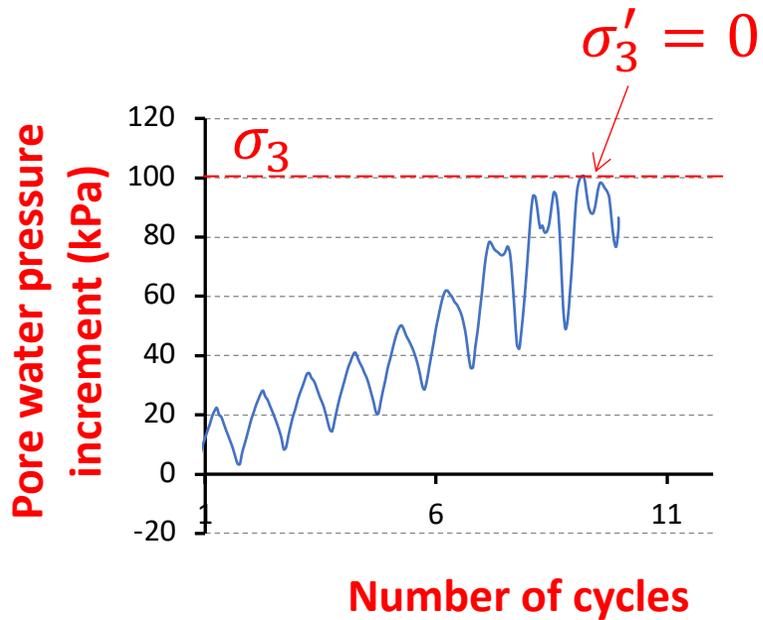
Unsaturated sample
 $B = 0.2$



Results

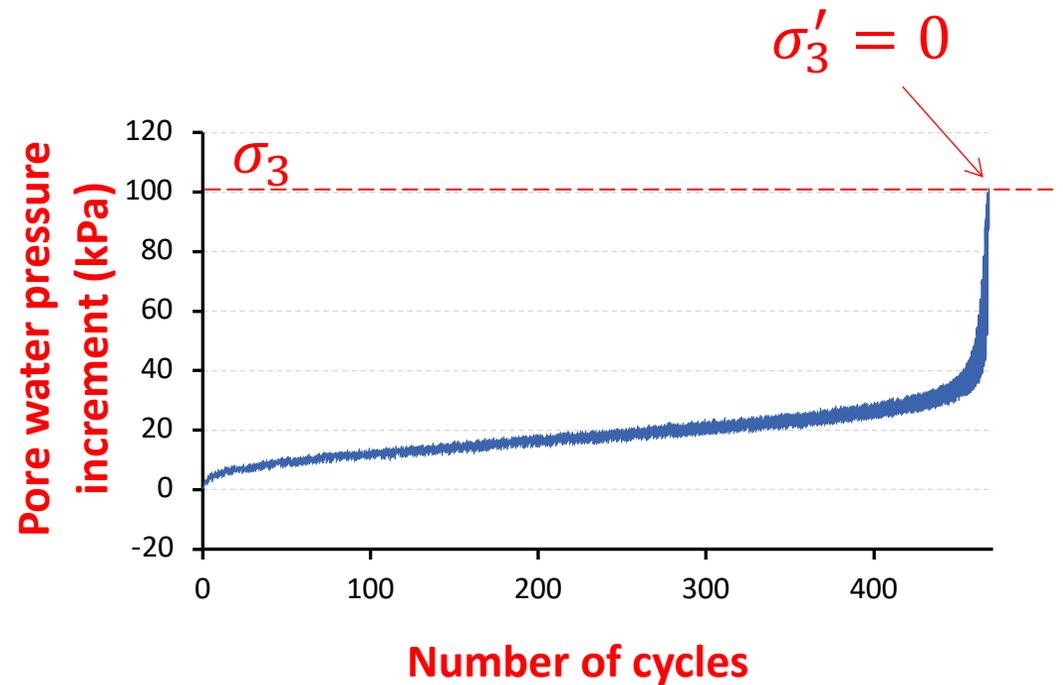
Saturated sample

$B = 0.97$

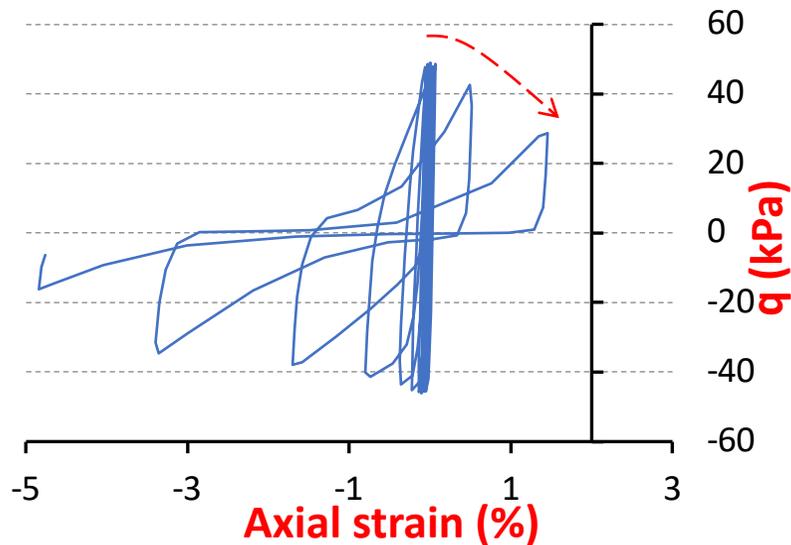


Unsaturated sample

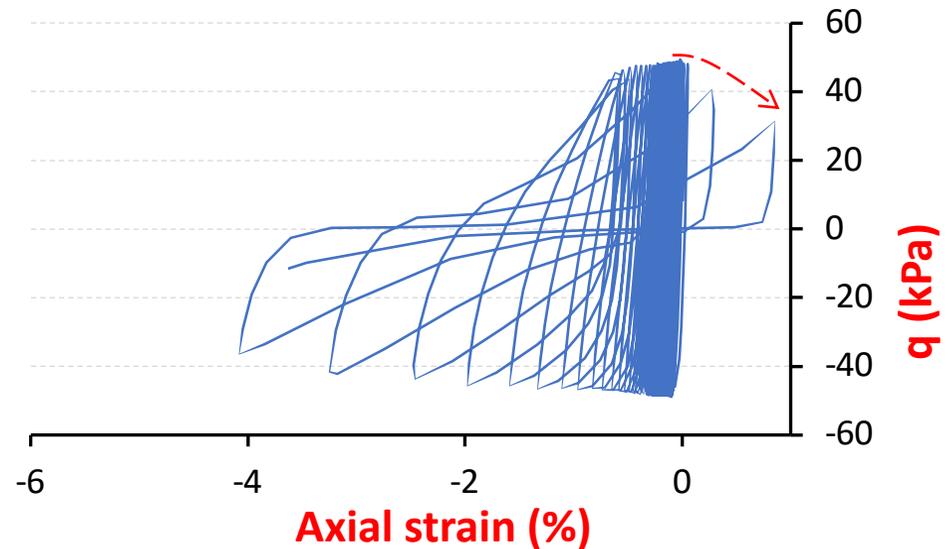
$B = 0.2$



Saturated sample
 $B = 0.97$



Unsaturated sample
 $B = 0.2$



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Conclusions

- Although the sample was not well saturated, it has been liquefied.
- The increase of pore fluid compressibility represented by B value results in the increase of number of cycles to liquefy the sample.
- The saturation degree affects significantly the liquefaction potential of soil
- The results of this study suggest that desaturation can be used as a sustainable method to improve soil properties to mitigate the effect of liquefaction.

Introduction and motivation	Material, apparatus, test procedure	Results	Conclusion
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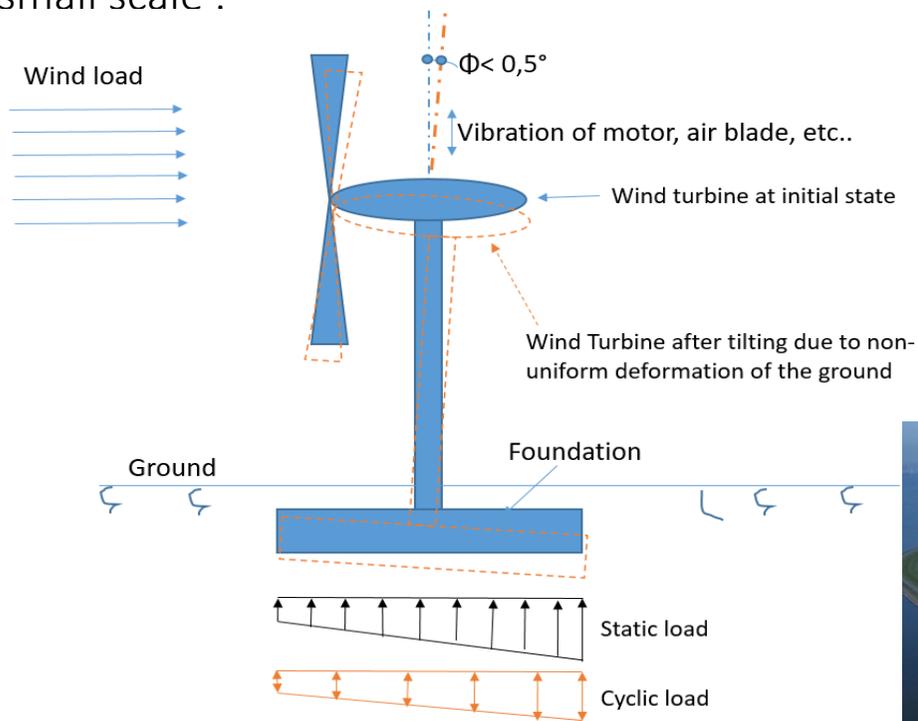
Thank you for your attention

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Experiences of liquefaction

- The sudden lost of soil strength
- Increase of pore water pressuse
- Damages significantly the site on large scale or on small scale .



Under wind turbine



Earthquake in Palu Indonesia 2018

